

A dynamic splash of clear blue water against a white background, with a thick blue horizontal bar at the bottom.

# Water and Sewer Tunnel Project Status and Conner Freud Pump Stations

*Todd King, P.E.*

*Field Services Director*



# Agenda

- 💧 DB-226 Detroit River Interceptor Project
- 💧 DB-150 Raw Water Tunnel Project
- 💧 CS-120 Conner Freud Pumping Stations



# DB-226 Detroit River Interceptor Project

CIP No: 222002

Start: 5/24/2018

Duration: Five Years

Project Delivery: Design Build

Project Team: Jay Dee – Contractor and Prime; FK Engineers – Designer of Record;  
Subconsultants = Applied Sciences, Inc. and Anderson, Eckstein and Westrick, Inc.

GLWA PM: Mini Panicker, P.E.

Scope: Evaluation and repair of the Detroit River Interceptor (DRI) sewer from Alter Road (City of Detroit border) to the WRRF. Approximately 12 miles of sewer with diameters from 8 to 16 feet.

Procurement Method: Quality Based Selection

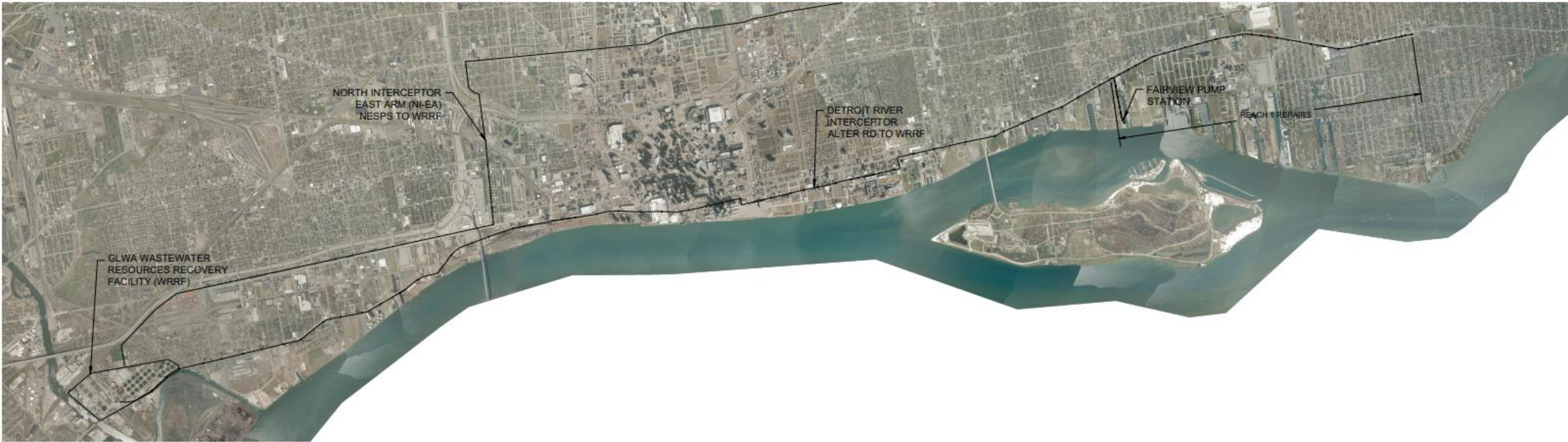
Original Contract Upper Limit: \$19.8 million

Original/Current Budget Estimate: \$29 million / \$ 50 to 60 million (20% contingency)

Current Estimated Duration: Six to Seven Years



# Project Scope – Divided into 3 reaches ~ 13 miles



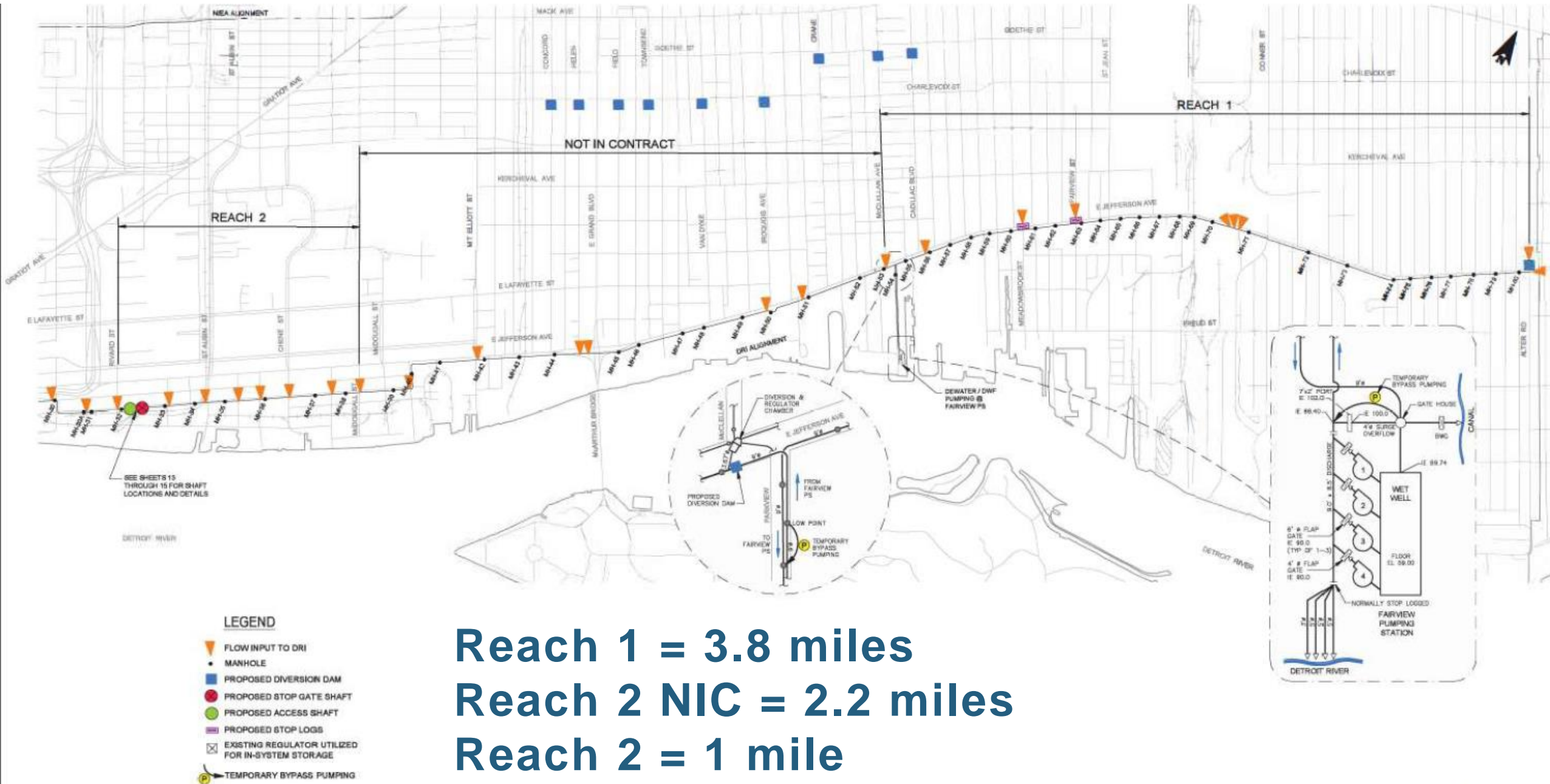


FIGURE NO. 1





# DB-226 Detroit River Interceptor Project (cont.)

## Engineering Budget:

\$3.4 million as bid

## Guaranteed Maximum Price (GMP) Negotiations:

Reach 1: \$5.8 million – 9/5/2018

Reach 2: \$4.7 million – 1/25/2019

Access Shafts: \$6.0 million – 1/25/2019

NIEA-DRI Tunnel: \$14 million -

## Added Scope:

Reach 1: Conner Creek Access Shafts + Manhole Improvements: \$1.3 million

Reach 1: Increased Repair quantities: \$1.5 million

Reach 2: Increased Estimated quantities: \$ 2.4 million

Reach 2: Area originally not in contract: \$6 million

Reach 3: Enhanced Access Shafts/Gates: \$3 million

Reach 3: Additional flows and higher than anticipated PS-1 Wet Level: \$12 million

Reach 3: Contingency for additional quantities for Reach 3: \$3.9 - \$13.9 million

Current Estimated Budget: \$50 – 60 million

# Significant opportunity for cost savings

DRI to North Interceptor East Arm (NIEA) Crossover

## 1. Features

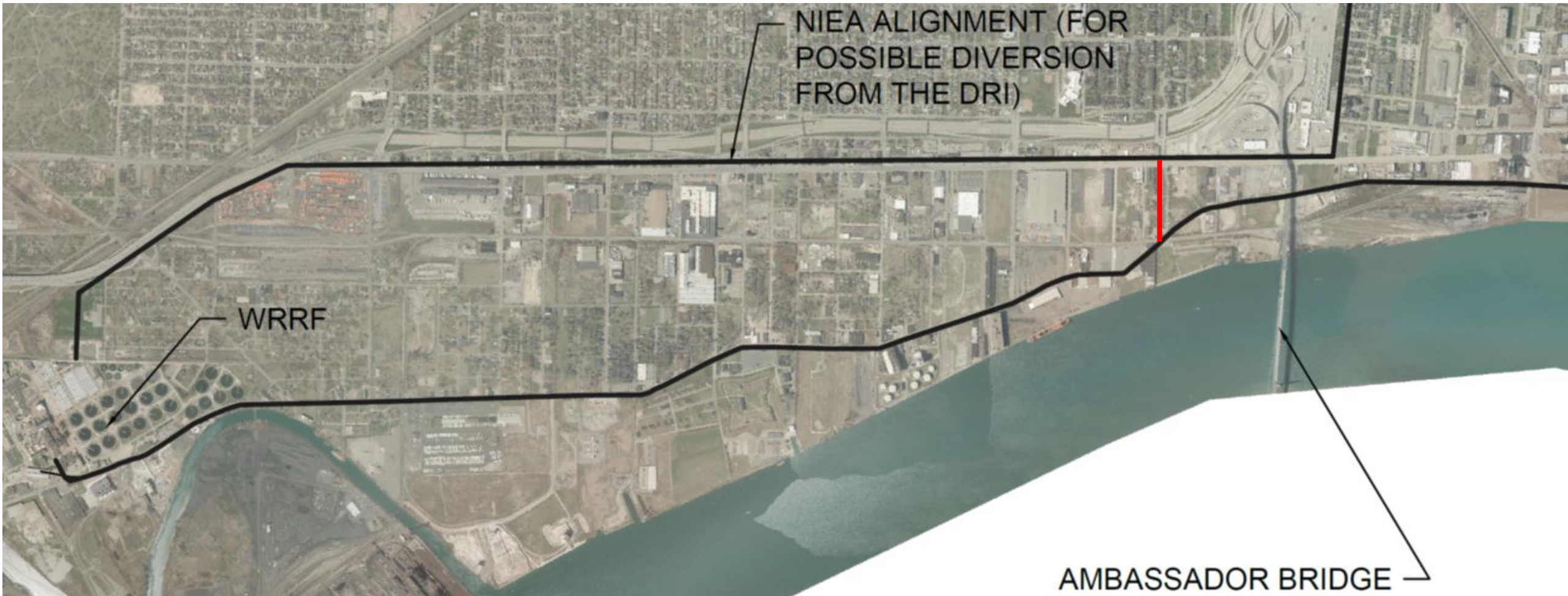
- Conveys dry weather flow from DRI to NIEA
- 84-inch tunnel to connect DRI and NIEA
- Tunnel boring machine for 980 lineal feet
- Connect to five DWSD lateral sewers

## 2. Benefits

- Eliminates significant portions of bypass pumping and delays from Fairview project
- Less disruption to WRRF with reduction in bypass pumping
- Reduces Reach 3 depths and velocities



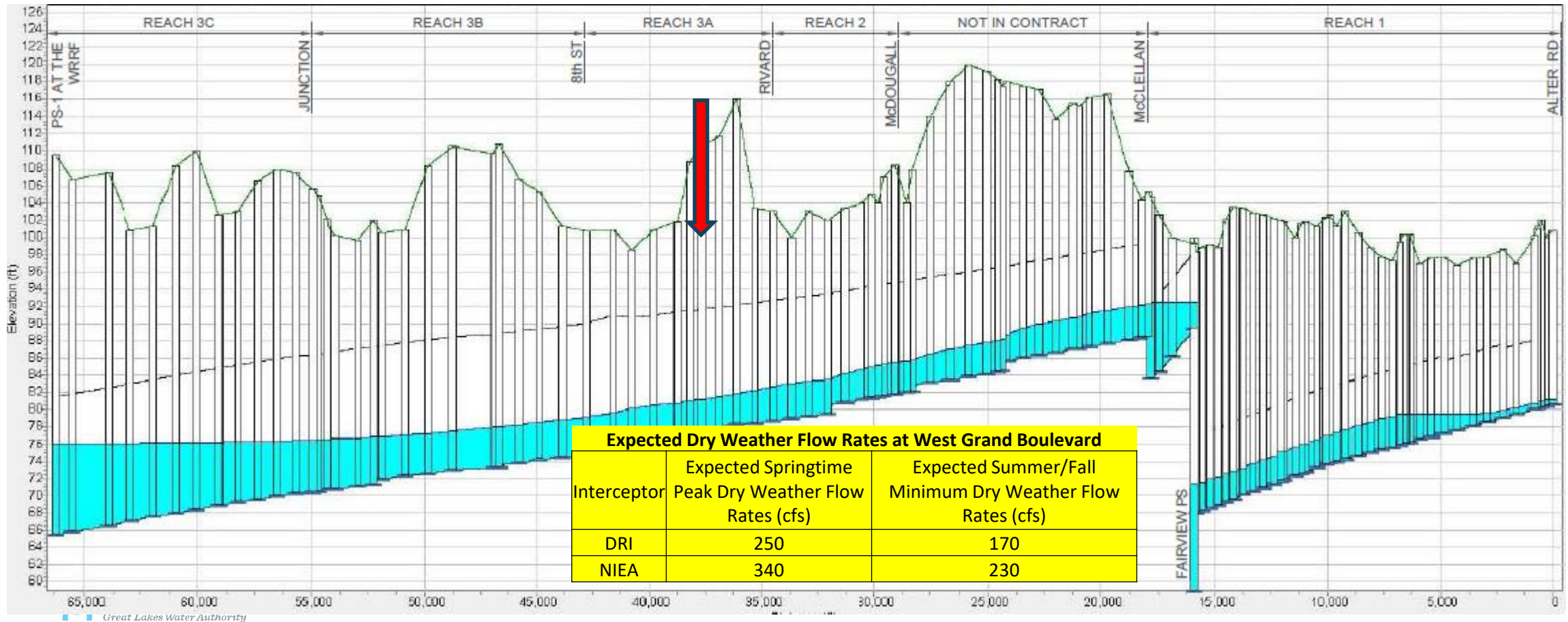
# DRI-NIEA Crossover



# DRI and NIEA Hydraulic Capacities

DRI: 15'-6"Ø at W. Grand Blvd. in Jefferson Ave. and capacity is 952 cfs.

NIEA: 13'-6"Ø at W. Grand Blvd. along Fort St. and capacity is 1,043 cfs.



# DB-226 Next Steps

- 1) EGLE and City of Detroit have approved DRI-NIEA Crossover in Principle
- 2) Need to negotiate a change order and GMP for the first phase of Reach 3 work to include the DRI-NIEA Crossover
- 3) Will present to Board as Change Order No. 1 for DB-226 by fall

# DB-150 Raw Water Tunnel Project

CIP No: 116002

Start: 1/29/2018

Duration: One Year to GMP, Construction TBD

Project Delivery: Progressive Design Build

Project Team: Ballard Marine – Contractor and Prime; Brierly and Associates – Designer of Record;

GLWA PM: Todd King, P.E.

Scope: Evaluation and repair of the raw water tunnels near Springwells WTP, Northeast WTP, and the combined Pennsylvania Tunnel that feeds same. This initial project includes supplemental investigation and production of 30-percent design to facilitate negotiation of GMP.

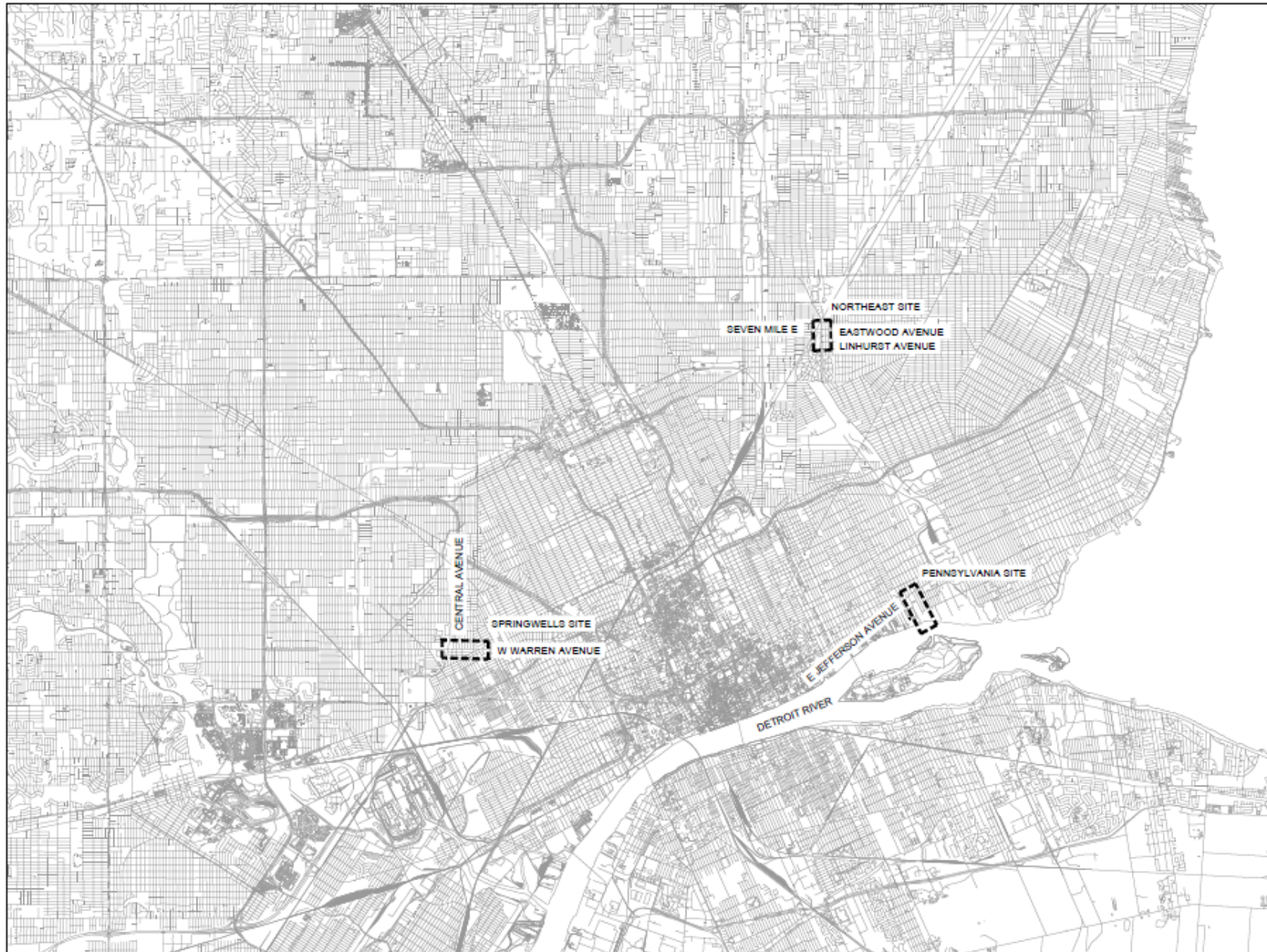
Procurement Method: Quality Based Selection

Original Contract Upper Limit: \$10.7 million

Original/Current Budget Estimate: \$30 million / \$67 to 105 million (50% contingency)

Current Estimated Duration: Five Years





**PROJECT KEY PLAN**  
SCALE: NTS



**PENNSYLVANIA SITE KEY PLAN**  
SCALE: NTS



**SPRINGWELLS SITE KEY PLAN**  
SCALE: NTS



**NORTHEAST SITE KEY PLAN**  
SCALE: NTS



Diver Tunnel Inspection/Mapping Sheet

Tunnel: Pennsylvania

Start Station: 105+75 (center of shaft)

End Station: 107+00

Inspection Date/Time: 10/9-10/10 @ 18:00

Video File Name: \_\_\_\_\_

Video Start Time: \_\_\_\_\_

Video End Time: \_\_\_\_\_

|                  |        | Anomaly Photo | Tunnel Ovality                                                                                                                                            |                  |        | Anomaly Photo | Tunnel Ovality                                                                                                                                            |
|------------------|--------|---------------|-----------------------------------------------------------------------------------------------------------------------------------------------------------|------------------|--------|---------------|-----------------------------------------------------------------------------------------------------------------------------------------------------------|
| <b>Anomaly 1</b> |        |               | <b>Fitted Ellipse at Pennsylvania 106+00</b><br><br>Hole Diam (ft): 14.00<br>Vert Diam (ft): 14.00<br>Ovality (%): 0.000<br>Max Bottomed Depth (in): 2.49 | <b>Anomaly 2</b> |        |               | <b>Fitted Ellipse at Pennsylvania 106+20</b><br><br>Hole Diam (ft): 14.00<br>Vert Diam (ft): 14.10<br>Ovality (%): 0.854<br>Max Bottomed Depth (in): 6.08 |
| Start Station    | 106+05 |               |                                                                                                                                                           | Start Station    | 106+20 |               |                                                                                                                                                           |
| End Station      | 106+05 |               |                                                                                                                                                           | End Station      | 106+20 |               |                                                                                                                                                           |
| <b>Anomaly 3</b> |        |               | <b>Fitted Ellipse at Pennsylvania 106+50</b><br><br>Hole Diam (ft): 14.40<br>Vert Diam (ft): 14.11<br>Ovality (%): 0.965<br>No Significant Sediment       | <b>Anomaly 4</b> |        |               | <b>Fitted Ellipse at Pennsylvania 106+70</b><br><br>Hole Diam (ft): 13.80<br>Vert Diam (ft): 14.10<br>Ovality (%): 0.18<br>No Significant Sediment        |
| Start Station    | 106+50 |               |                                                                                                                                                           | Start Station    | 106+70 |               |                                                                                                                                                           |
| End Station      | 106+50 |               |                                                                                                                                                           | End Station      | 106+70 |               |                                                                                                                                                           |
| <b>Anomaly 5</b> |        |               | <b>Fitted Ellipse at Pennsylvania 107+30</b><br><br>Hole Diam (ft): 13.00<br>Vert Diam (ft): 12.88<br>Ovality (%): 0.08<br>No Significant Sediment        | <b>Anomaly 6</b> |        |               | <b>Fitted Ellipse at Pennsylvania 107+40</b><br><br>Hole Diam (ft): 13.00<br>Vert Diam (ft): 14.00<br>Ovality (%): 0.34<br>No Significant Sediment        |
| Start Station    | 107+30 |               |                                                                                                                                                           | Start Station    | 107+40 |               |                                                                                                                                                           |
| End Station      | 107+30 |               |                                                                                                                                                           | End Station      | 107+40 |               |                                                                                                                                                           |
| <b>Anomaly 7</b> |        |               | <b>Fitted Ellipse at Pennsylvania 107+60</b><br><br>Hole Diam (ft): 13.90<br>Vert Diam (ft): 14.45<br>Ovality (%): 0.14<br>No Significant Sediment        | <b>Anomaly 8</b> |        |               | <b>Fitted Ellipse at Pennsylvania 107+80</b><br><br>Hole Diam (ft): 13.0<br>Vert Diam (ft): 14.00<br>Ovality (%): 0.35<br>No Significant Sediment         |
| Start Station    | 107+60 |               |                                                                                                                                                           | Start Station    | 107+80 |               |                                                                                                                                                           |
| End Station      | 107+60 |               |                                                                                                                                                           | End Station      | 107+80 |               |                                                                                                                                                           |





Form No. TME-C42  
 Revision No. 0  
 Revision Date: 11/30/2018

## OBTAINING AND TESTING DRILLED CORES AND SAWED BEAMS OF CONCRETE

Quality Assurance

S&ME, Inc. - Columbus 6190 Enterprise Court, Dublin, Ohio 43016

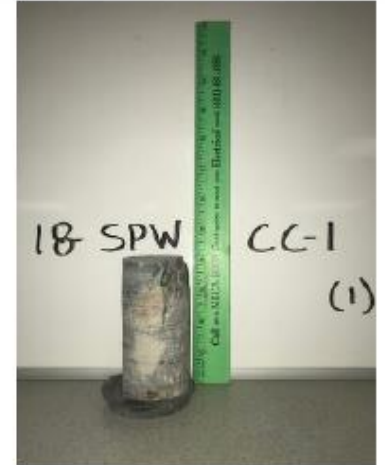
Project No.: 1117-18-022 Date Photos Taken: 12/3/2018

Project Name: GLWA Detroit Raw Water Tunnels Log No:

Description: Concrete Cores

Specimen No.: 18-SPW CC-1  
 Sample 1

Compressive Strength: 6379 psi



Specimen No.: 18-SPW CC-1  
 Sample 2

Compressive Strength: 8424 psi

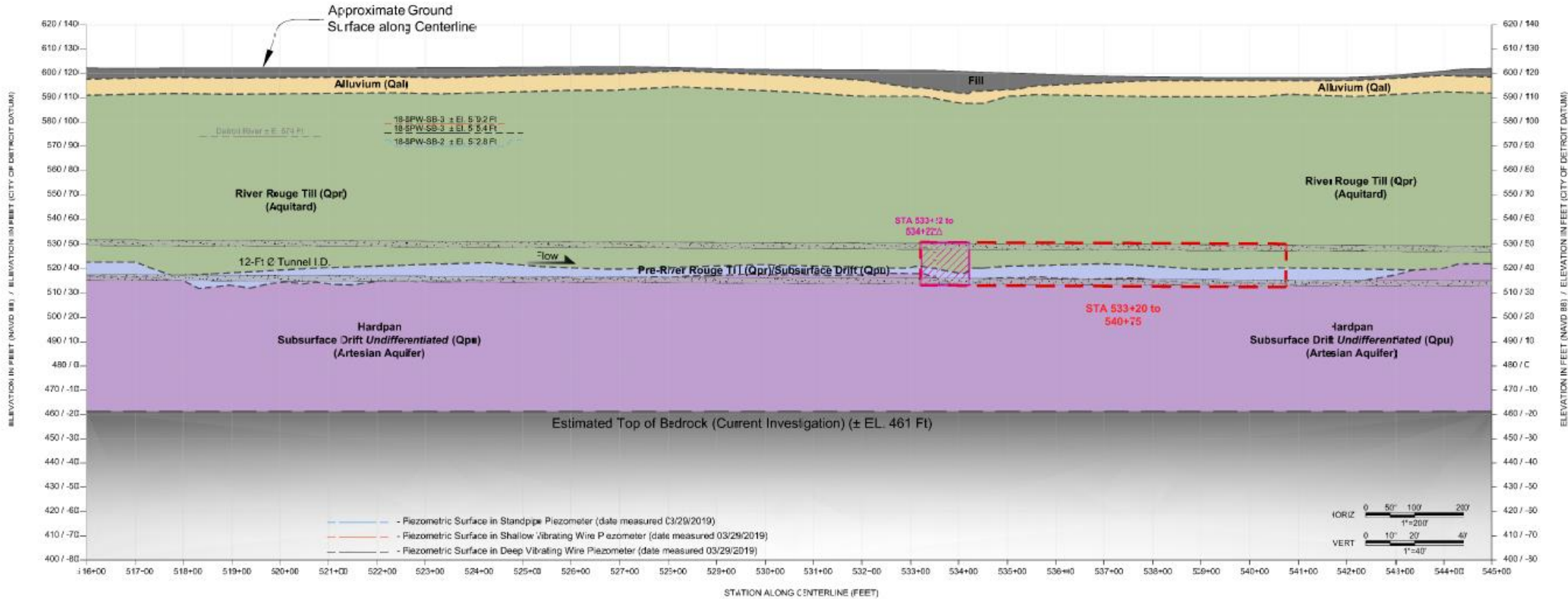


Paula J. Manning  
 Performed By

11/30/2018  
 Date

Erica Hoodyear  
 Checked By

12/4/2018  
 Date



**Notes**  
 Profile represents generalized conditions based on test borings and field observations. Formation contacts and other boundaries as shown do not imply exact locations nor that the portrayed formations are necessarily continuous between borings. Borings drilled in 1928 by Ohio Drilling Company & Pennsylvania Drilling Company, borings drilled in 2016 by DLZ - American Drilling, Inc., NAVD 83 elevation has a correction of +429.233 ft from the Detroit Datum at the Springwells WTP.

- Stratigraphy**
- Fill
  - Alluvium (Qal)
  - River Rouge Till (Qpr)
  - Pre-River Rouge Till (Qpr)/Subsurface Drift Undifferentiated (Qpp)
  - Hardpan Subsurface Drift Undifferentiated (Qpu)

- Legend**
- - - Piezometric Surface in Standpipe Piezometer (date measured 03/29/2019)
  - - - Piezometric Surface in Shallow Vibrating Wire Piezometer (date measured 03/29/2019)
  - - - Piezometric Surface in Deep Vibrating Wire Piezometer (date measured 03/29/2019)
  - - - Area of Contracted Portion of Tunnel
  - - - Area of Distressed Tunnel Section Identified in Video Review
  - Δ denotes area to be confirmed

**BRIERLEY ASSOCIATES**  
 Creating Space Underground  
 15808 Ranch Fd 620 N, Ste 210, Austin, TX 78717  
 PHONE: 512.219.1733 FAX: 512.219.1759

**CLIENT** Great Lakes Water Authority  
 735 Randolph Street Suite 1900  
 Detroit, MI 48226

**PROJECT NUMBER** GLWA-DB-150, Design-Build  
 717002-030

**TITLE** SPRINGWELLS TUNNEL  
 STRATIGRAPHIC CONDITIONS

**WORK/PLAT** TM-GEO-1.4



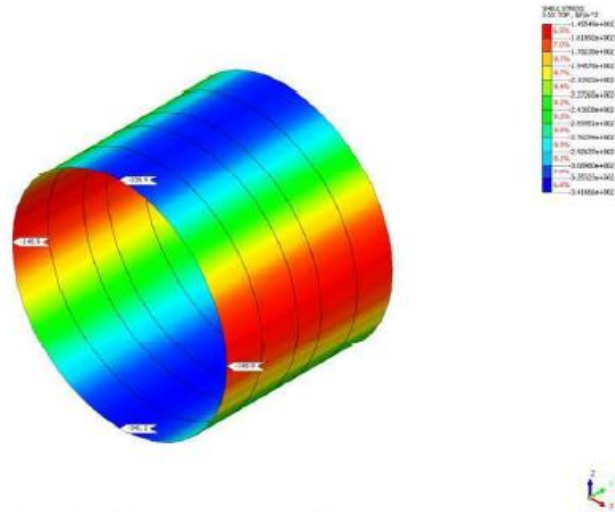


Figure 19: CIP Liner Circumferential Stress Outside Face – Tunnel Dewatered

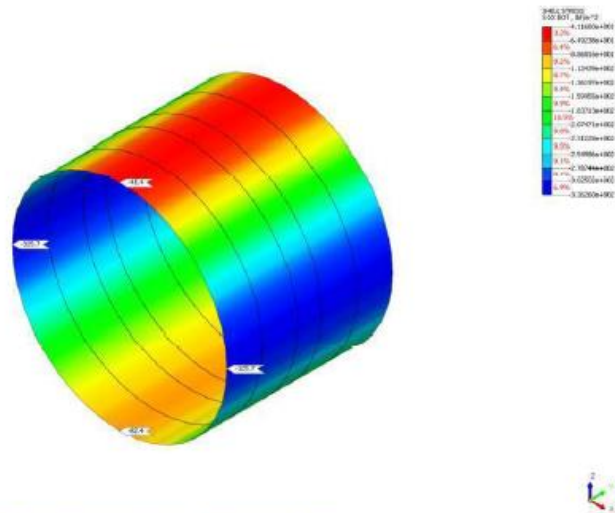


Figure 20: CIP Liner Circumferential Stress Inside Face – Tunnel Dewatered

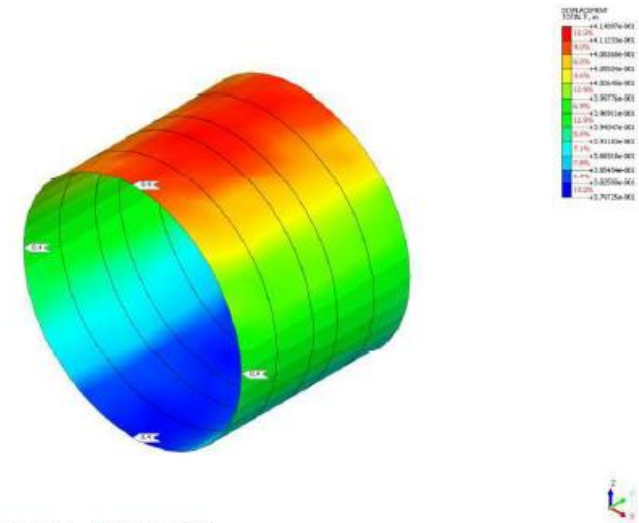


Figure 13: CIP Liner Total Deformation – Tunnel In-service

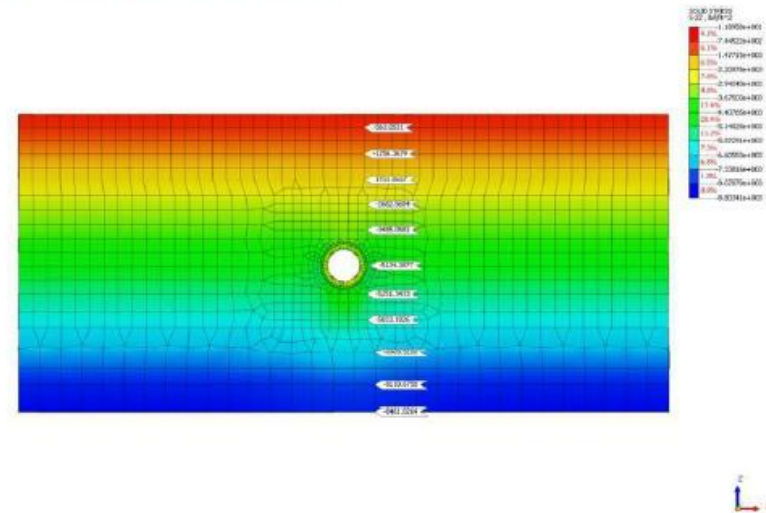


Figure 14: Vertical Effective Stress – Tunnel Dewatered

# DB-150 Significant Variations

| Scope Item                  | Original RFP              | 30 % BOD          | Estimated Costs        |
|-----------------------------|---------------------------|-------------------|------------------------|
| Springwells Tunnel Repairs  | 270 LF (in two locations) | 755 LF (combined) | \$40.6 to 60.9 million |
| Pennsylvania Tunnel Repairs | 225 LF                    | 225 LF            |                        |
| Access Shafts               | Temporary                 | Permanent         |                        |
| Northeast Tunnel            | 400 LF                    | 500 LF            | \$26.4 to 39.6 million |

# DB-150 Current GMP Status

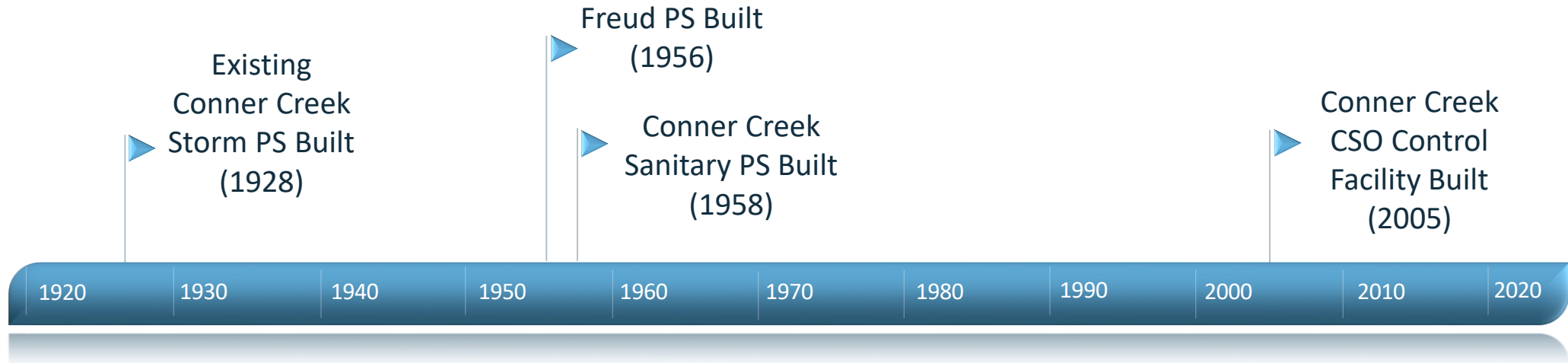
- 1) Base Contract with Stainless Steel Flexible Liners at SPW and NE, Crack Repair at PENN = \$80.5 million
- 2) GLWA Counteroffer = \$66 million
- 3) Deduct for no action at NE tunnel = \$23.9 million
- 4) Value of NE Tunnel as asset approximately \$400 million
- 5) Incurred to date-additional investigation and 30-percent design = \$10 million

# CS-120 Conner and Freud Storm and Sanitary Pump Stations

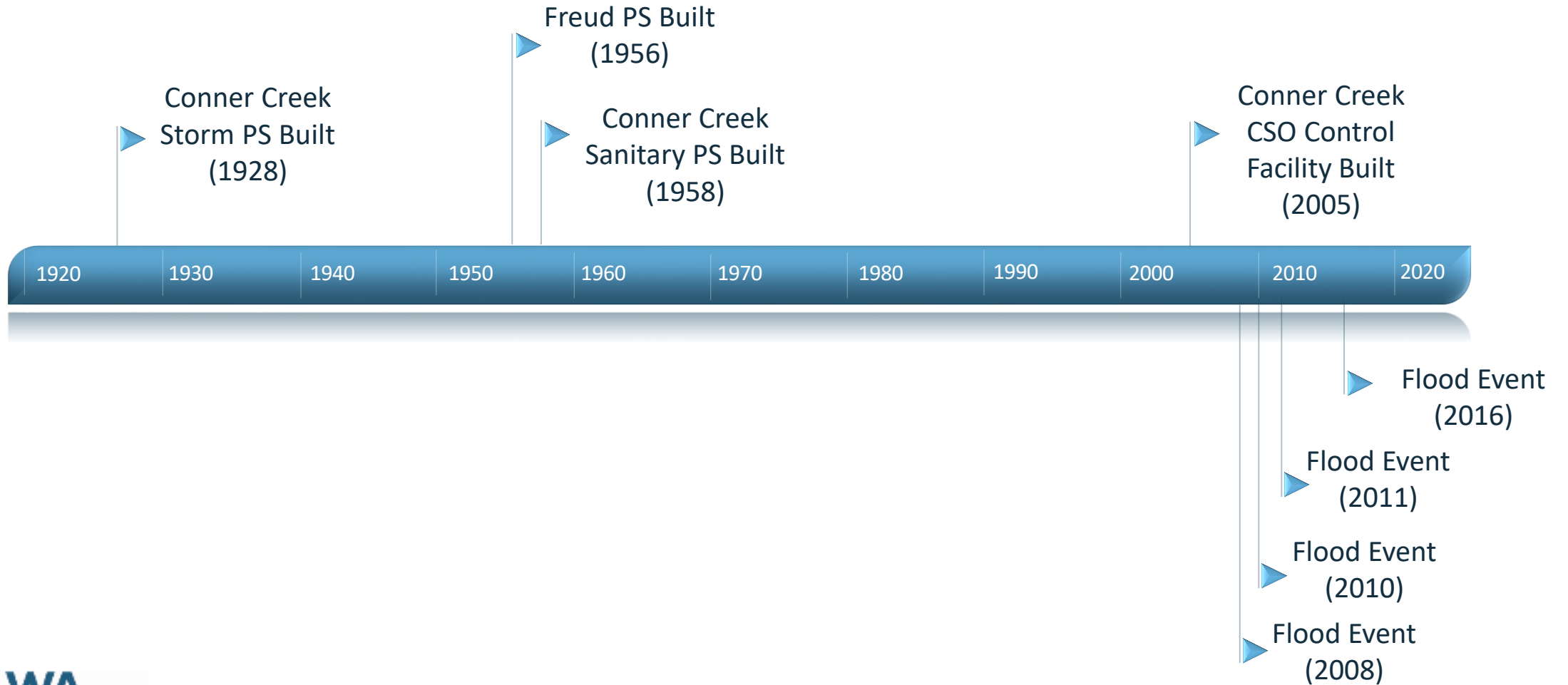
CIP No: 232002  
Start: 2016  
Duration: Five Years  
Project Delivery: Design Bid Build  
Project Team: Arcadis, & Brown and Caldwell;  
GLWA PM: Mini Panicker, P.E.  
Scope: Evaluation and upgrade of Conner and Freud Sanitary and Storm Pump Stations,  
each with 2 BGD capacity  
Procurement Method: Quality Based Selection – Design  
Original Contract Upper Limit: \$4.4 million  
Incurred to Date: \$1.2 million  
Original/Current/Potential Budget Estimate: \$22.5 million / \$ 160 million / \$218 million  
Current Estimated Duration: Eight Years



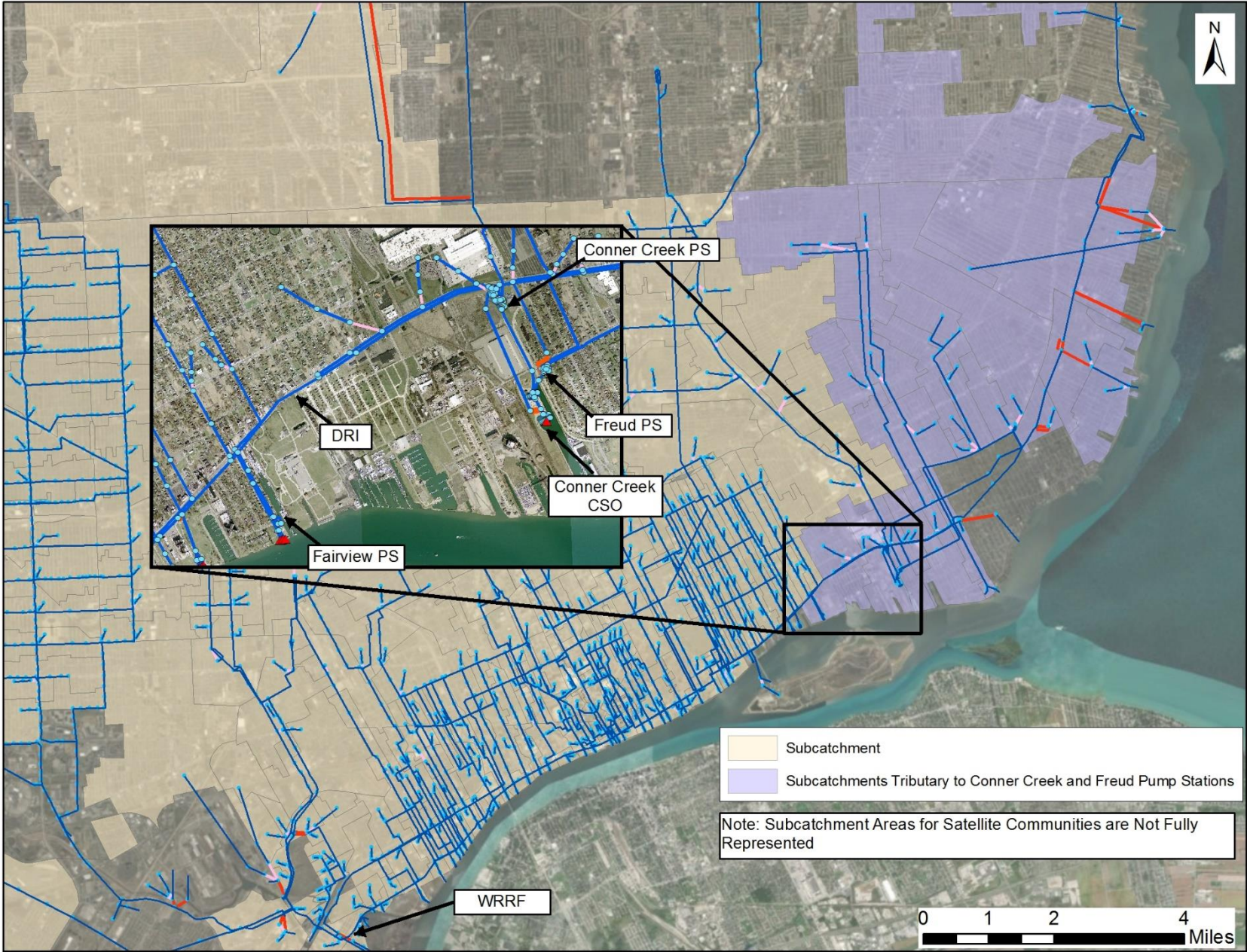
# Timeline – Pump Stations



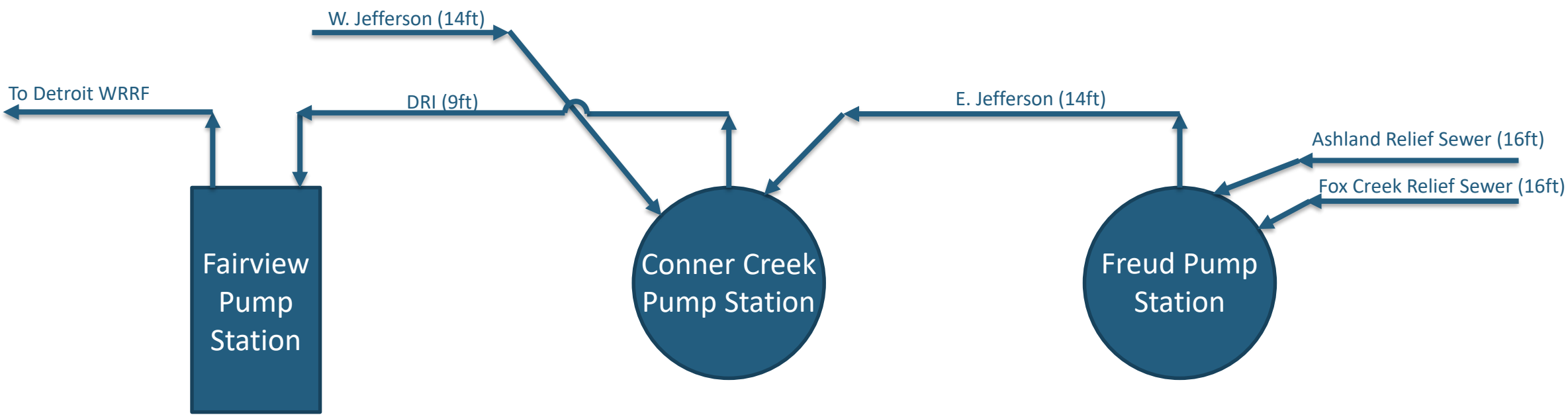
# Timeline – Recent Wet Weather Events



# Service Area

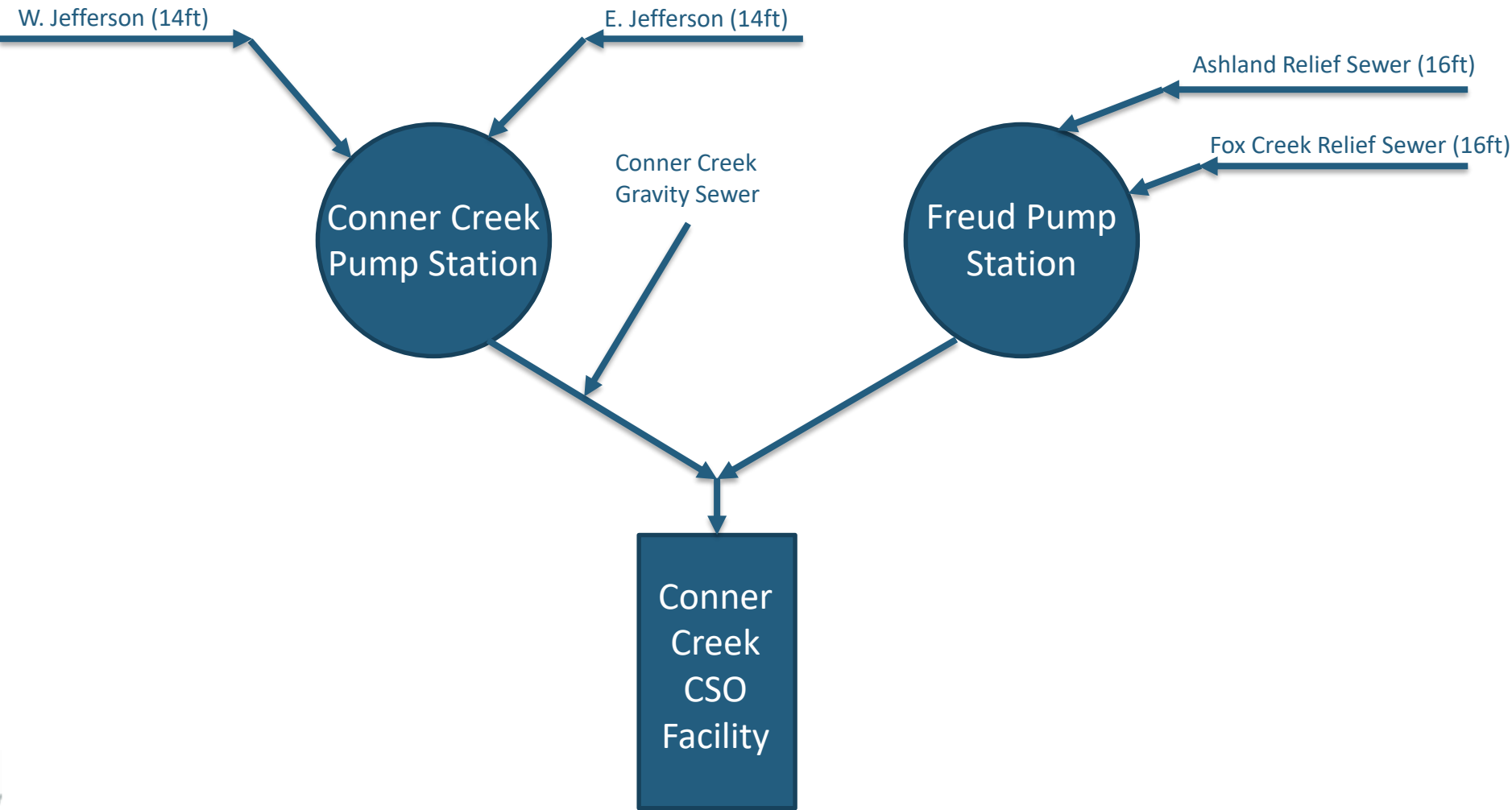


# Overview – Existing Dry Weather Flow



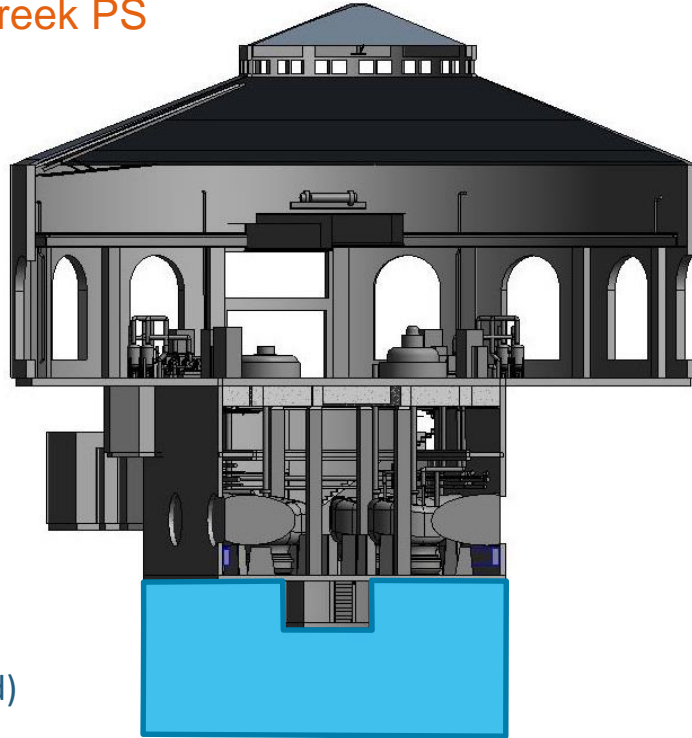


# Overview – Existing Wet Weather Flow



# Overview – Existing Pump Stations

Conner Creek PS

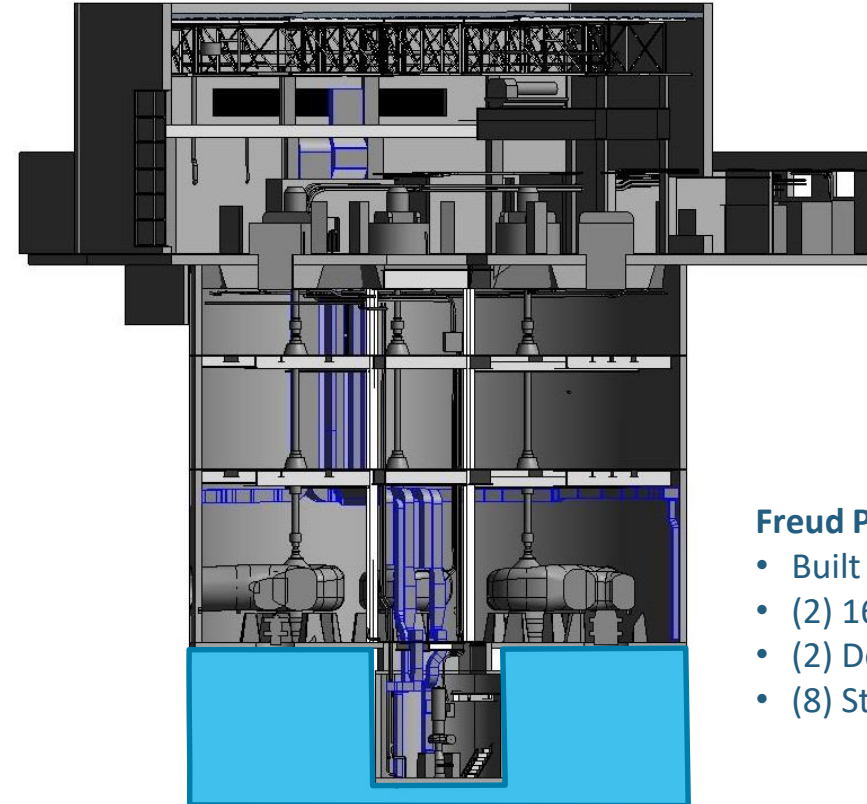


Storm Wet Well  
Floor – El. 55

## Conner Creek PS

- Built in 1928 (91 yrs old)
- (2) 14' Influent Sewers
- (4) Sanitary Pumps (144 MGD)
- (8) Storm Pumps (2.23 BGD)

Freud PS



Storm Wet Well  
Floor – El. 20

## Freud PS

- Built in 1954 (65 yrs old)
- (2) 16' Influent Sewers
- (2) Dewatering Pumps (13 MGD)
- (8) Storm Pumps (2.03 BGD)

# Project Purpose

“The primary objective of this project is to study the overall performance of both the pumping stations and develop and design an operational strategy to optimize the utilization of interconnected piping and operation between these two pumping stations and the Conner Creek Retention and Treatment Basin.” (Original RFP - December 2016)

# 2017 Scope of Work

## Improvements for Existing Pump Stations

- Optimize Operating and Control Strategy for Freud PS, Conner Creek PS, and Conner Creek CSO Facility
- Optimize Conner Storm Pump Priming
- Incorporate means to isolate wet wells
- Evaluate hydraulic modifications to existing Storm and Sanitary Wet Wells
- Assess Physical Condition of each Pump Station
- Improve Equipment Handling

# Work Completed To-Date

| Task                                        | Conclusions                                                                                                                                                                                                                                                                                                        |
|---------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| Physical Condition Assessment               | Satisfactory considering age of equipment and structures; repairs and upgrades as expected                                                                                                                                                                                                                         |
| Physical Modeling – Existing System         | Conner: Confirms operational challenges related to turbulence and air entrainment for both Sanitary and Storm Pump Stations<br>Freud: Confirms Sanitary hydraulic challenges with existing design ranges                                                                                                           |
| Hydraulic Modeling – Existing System        | Confirms surcharging challenges in system                                                                                                                                                                                                                                                                          |
| Pumping Hydraulics and Operation Assessment | Poor: <ul style="list-style-type: none"><li>• Conner Storm – shallow wet well and extremely tight operating range, very close to basement flooding elevations</li><li>• Conner Sanitary – undersized capacity</li><li>• Freud Sanitary – undersized capacity and originally intended for dewatering only</li></ul> |



# Concept Alternatives – Initial

## Alt 1 - Minimum Improvements for Conner Creek PS and Freud PS

- Conner Storm: 8 New vertical column pumps (replace existing)
- Conner Sanitary: Rehab existing 4 pumps; add 2 more pumps in new wet well
- Freud Storm: Rehab existing 8 pumps
- Freud Sanitary: Replace existing Dewatering Pumps

## Alt 2 - New Conner Creek PS and Intermediate Improvements for Freud PS

- Conner Storm: New deep 2.2 BGD Pump Station
- Conner Sanitary: New deep 200 MGD Pump Station
- Freud Storm: Rehab existing 8 pumps
- Freud Sanitary: New 30 MGD Pump Station and replace existing Dewatering Pumps

## Alt 3 - New Combined Pump Station

- Combined 4 BGD Pump Station with tunneling to connect 4 influent sewers

# Concept Alternatives – Value Engineering Input

General Consensus with the Value Engineering Team:

- Conner Sanitary – New Pump Station (200 MGD)
- Freud Storm – Replace 2 Dewatering Pumps; Rehab 8 Storm Pumps
- Freud Sanitary – New Pump Station (30 MGD)

Estimated combined capital cost – \$53 million

# Concept Alternatives – Value Engineering Input

## Conner Storm – Initial Alternatives:

- Alt 1 - 8 New vertical column pumps (replace existing)
- Alt 2 - New deep 2.2 BGD Pump Station

## Conner Storm – VE Team Suggested Alternatives:

- Alt 1a – Supplemental wet well with two new storm pumps
- Alt 1b – New 1 BGD capacity pump station and improve priming system on existing storm pumps
- Alt 1c – New 2.2 BGD capacity pump station as described under Alternative 2 but initially with 1 BGD of pumping capacity; utilize existing station for remaining life and add new pumps as existing pumps fail

# Alternatives Comparison Summary – Conner Storm

|                                             | Alt 1<br>New Pumps<br>Existing PS | Alt 1a<br>New Pumps<br>Existing PS +<br>Small New PS | Alt 1b<br>New 1 BGD PS<br>+ Existing PS | Alt 1c<br>New 2.2 BGD<br>PS with<br>1/2 Pumps +<br>Existing PS | Alt 2<br>New 2.2 BGD<br>PS |
|---------------------------------------------|-----------------------------------|------------------------------------------------------|-----------------------------------------|----------------------------------------------------------------|----------------------------|
| All pumps rapid start (no vacuum priming)   | ✓                                 | ✓                                                    |                                         |                                                                | ✓                          |
| Pumping Capacity ( 2.2 BGD)                 | *                                 | ✓                                                    | ✓                                       | ✓                                                              | ✓                          |
| Enhanced wet well operability               |                                   |                                                      | ✓                                       | ✓                                                              | ✓                          |
| Conforms to Hydraulic Institute standards** |                                   |                                                      |                                         |                                                                | ✓                          |
| Ability to isolate wet well for maintenance |                                   |                                                      | ✓                                       | ✓                                                              | ✓                          |
| Capital Cost (\$millions)                   | 76                                | 104                                                  | 118                                     | 135                                                            | 160                        |
| Net Present Value – 40 year (\$millions)    | 151                               | 185                                                  | 204                                     | 265                                                            | 231                        |

\* TBD based on additional physical and CFD modeling

\*\* Maximizes long term investment value, e.g. pumps and piping last longer with fewer problems

# Alternatives Comparison Summary – Conner Storm

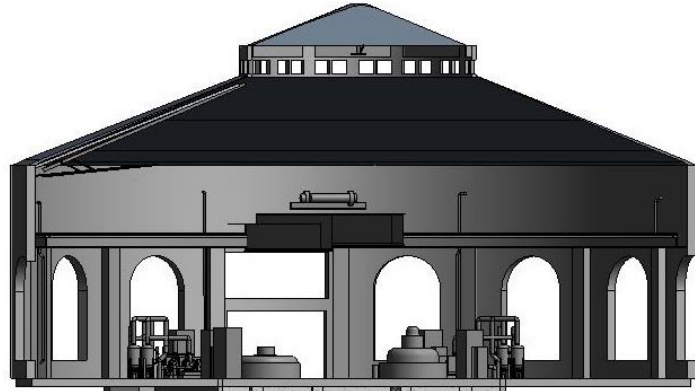
|                                                               | Alt 1<br>New Pumps<br>Existing PS | Alt 1b<br>New 1 BGD PS +<br>Existing PS | Alt 2<br>New 2.2 BGD PS |
|---------------------------------------------------------------|-----------------------------------|-----------------------------------------|-------------------------|
| No additional modeling required to prove concept              |                                   | ✓                                       | ✓                       |
| Construction duration – estimate                              | 6 years                           | 4 years                                 | 5 years                 |
| Lower constructability / regulatory risks during construction |                                   | ✓                                       | ✓                       |
| Firm Pumping Capacity (2.2 BGD)                               | *                                 | ✓                                       | ✓                       |
| Does not require land acquisition                             | ✓                                 |                                         |                         |
| Provisions to facilitate equipment removal                    | Limited                           | Limited                                 | All                     |
| Capital Cost (\$millions)                                     | 76                                | 118                                     | 160                     |
| Net Present Value – 40 year (\$millions)                      | 151                               | 204                                     | 231                     |

\* TBD based on additional physical and CFD modeling

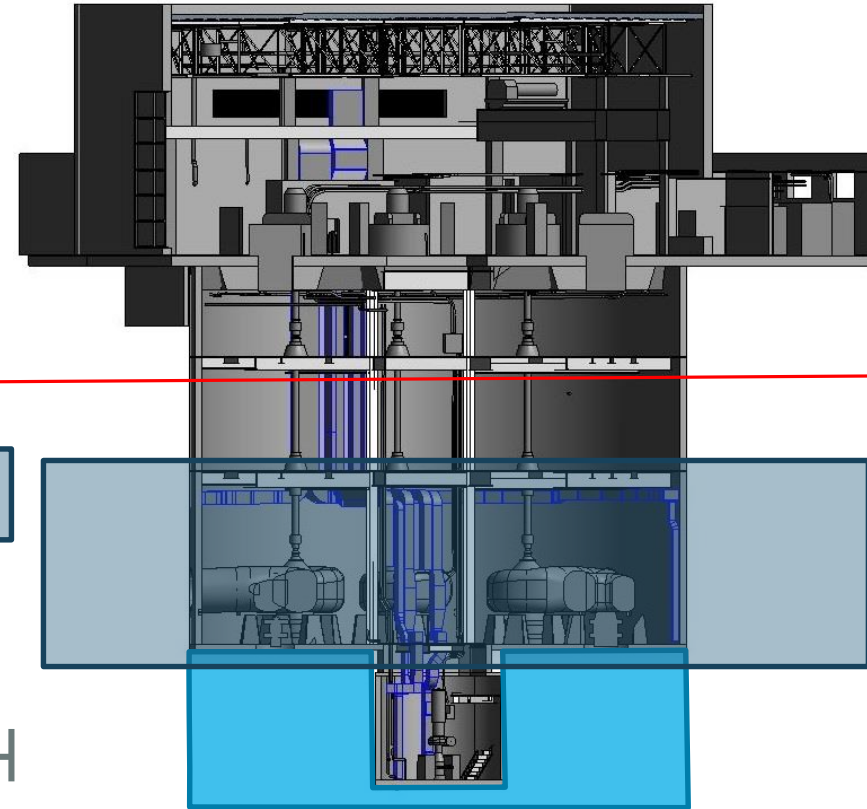


# Alternatives Comparison Graphic – Conner Storm

Existing



New



Approximate  
Basement Flood  
Elevation

Storm Pumping  
Range (14 ft)

Storm Pumping  
Range (>30 ft)

Storm Wet Well  
Floor – El. 55

Storm Wet Well  
Floor – El. 20

Alt 1 - New Pumps Existing PS

Alt 1b - New 1 BGD PS + Existing PS

Alt 2 - New 2.2 BGD PS

# Pro and Con – Alternative 1 – New Pumps in Old Conner Station

## Pro

- Least expensive
- Eliminates vacuum priming as potential cause of failure

## Con

- Does not meet Hydraulic Institute (HI) guidelines for pumps and wet well
- Requires additional, high level modeling (computational fluid dynamics and physical model)
- Prolongs schedule and increases demands on staff to mitigate risk of potential flooding events
- Does not provide capability to isolate wet well

# Pro and Con – Alternative 1b – New 1 BGD Pump Station and run existing Conner Station until failure

## Pro

- Best operational solution for least cost
- New Pump Station will meet HI guidelines
- Addition of 3<sup>rd</sup> storm pump station increases flexibility

## Con

- Existing pump station will not meet HI guidelines
- Retains complex vacuum priming
- Addition of 3<sup>rd</sup> storm pump station increases system complexity

# Pro and Con – Alternative 2 – New 2.2 BGD Pump Station

## Pro

- Meets HI guidelines
- Eliminates vacuum priming
- Shortest schedule
- Standardized equipment (improves operability)
- Increased isolation capability (improves maintainability)

## Con

- Most expensive
- Largest land acquisition footprint



# Alternatives Comparison Summary – Conner Storm

|                                          | Alt 1<br>New Pumps<br>Existing PS | Alt 1b<br>New 1 BGD PS +<br>Existing PS | Alt 2<br>New 2.2 BGD PS |
|------------------------------------------|-----------------------------------|-----------------------------------------|-------------------------|
| Least Cost                               | ✓                                 |                                         |                         |
| Least Time to Implement                  |                                   | ✓                                       |                         |
| Best Technical Solution                  |                                   |                                         | ✓                       |
| Least Property Acquisition               | ✓                                 |                                         |                         |
| Best Maintainability                     |                                   |                                         | ✓                       |
| Lowest Operating and Maintenance Cost    |                                   |                                         | ✓                       |
| Capital Cost (\$millions)                | 76                                | 118                                     | 160                     |
| Net Present Value – 40 year (\$millions) | 151                               | 204                                     | 231                     |

# Alternatives Comparison Summary – Conner Storm –

## Eliminating highest cost alternative

|                                          | Alt 1<br>New Pumps<br>Existing PS | Alt 1b<br>New 1 BGD PS +<br>Existing PS | <del>Alt 2<br/>New 2.2 BGD PS</del> |
|------------------------------------------|-----------------------------------|-----------------------------------------|-------------------------------------|
| Least Cost                               | ✓                                 |                                         | <del></del>                         |
| Least Time to Implement                  |                                   | ✓                                       | <del></del>                         |
| Best Technical Solution                  |                                   | ✓                                       | <del>✓</del>                        |
| Least Property Acquisition               | ✓                                 |                                         | <del></del>                         |
| Best Maintainability                     |                                   | ✓                                       | <del>✓</del>                        |
| Lowest Operating and Maintenance Cost    |                                   | ✓                                       | <del>✓</del>                        |
| Capital Cost (\$millions)                | 76                                | 118                                     | <del>160</del>                      |
| Net Present Value – 40 year (\$millions) | 151                               | 204                                     | <del>231</del>                      |

# Alternatives Comparison Summary – Conner Storm –

## Eliminating longest time alternative

|                                          | Alt 1<br>New Pumps<br>Existing PS | Alt 1b<br>New 1 BGD PS +<br>Existing PS | Alt 2<br>New 2.2 BGD PS |
|------------------------------------------|-----------------------------------|-----------------------------------------|-------------------------|
| Least Cost                               | ✓                                 | ✓                                       |                         |
| Least Time to Implement                  |                                   | ✓                                       |                         |
| Best Technical Solution                  |                                   |                                         | ✓                       |
| Least Property Acquisition               | ✓                                 | ✓                                       |                         |
| Best Maintainability                     |                                   |                                         | ✓                       |
| Lowest Operating and Maintenance Cost    |                                   |                                         | ✓                       |
| Capital Cost (\$millions)                | 76                                | 118                                     | 160                     |
| Net Present Value – 40 year (\$millions) | 151                               | 204                                     | 231                     |



# Best Technical Solution

## Alt 2 - New Conner Creek PS and Intermediate Improvements for Freud PS

- Conner Storm: New deep 2.2 BGD Pump Station
- Conner Sanitary: New deep 200 MGD Pump Station
- Freud Storm: Rehab existing 8 pumps
- Freud Sanitary: New 30 MGD Pump Station and replace existing Dewatering Pumps
- Total Capital Cost: \$220 million

## Why?

- Based on criticality and consequence of failure, Alt 2 aligns with GLWA's goals:
  - Lowest risk of flooding
  - Improve reliability and operability

# Plan Forward

GLWA Selection of Alternative

Amend existing contract CS-120

Proceed with Preliminary Design based on Recommended Alternative

Initiate land acquisition

Develop Basis of Design Report for both Pump Stations

Move into Final Design for two separate construction projects

# Estimated Schedule

GLWA Selection of Alternative – Winter, 2019/20

Preliminary Design completed – Fall 2020

Final Design Freud PS completed – Summer 2021

Final Design Conner Creek PS completed – Spring 2022

Construction Freud PS completed – Spring 2024

Construction Conner Creek PS completed – Spring 2026

# Estimated Fiscal Year Spend - OUTDATED

| FY Start | Total          | Freud         | Conner         |
|----------|----------------|---------------|----------------|
| 7/1/2019 | \$ 3,048,000   | \$ 1,252,000  | \$ 1,796,000   |
| 7/1/2020 | \$ 5,996,000   | \$ 1,700,000  | \$ 4,296,000   |
| 7/1/2021 | \$ 45,074,000  | \$ 7,812,000  | \$ 37,262,000  |
| 7/1/2022 | \$ 42,170,000  | \$ 7,908,000  | \$ 34,262,000  |
| 7/1/2023 | \$ 42,074,000  | \$ 7,812,000  | \$ 34,262,000  |
| 7/1/2024 | \$ 42,170,000  | \$ 7,908,000  | \$ 34,262,000  |
| 7/1/2025 | \$ 34,262,000  |               | \$ 34,262,000  |
| 7/1/2026 | \$ 3,700,000   |               | \$ 3,700,000   |
| Total    | \$ 218,494,000 | \$ 34,392,000 | \$ 184,102,000 |





**GLWA**

*Great Lakes Water Authority*

**Questions?**